



**REGIONAL DISTRICT  
of Fraser-Fort George**

Main Office: 155 George Street, Prince George, BC V2L 1P8  
Telephone: 250-960-4400 / Fax: 250-562-8676  
Toll Free: 1-800-667-1959 /  
<http://www.rdffg.bc.ca>

July 2, 2020

**ADDENDUM No. 1**

**Invitation to Quote CS-20-09  
NESS LAKE COMMUNITY HALL REAR ROOF EXTENSION**

*The addendum is being issued prior to the closing of the invitation to quote (ITQ) to provide further information, make changes to, or to clarify the ITQ documents and is to be read, interpreted and coordinated with all other parts of the ITQ documents. In the case of a conflict with the balance of the documents, this Addendum shall govern. **Bidders shall attach a signed copy of this addendum to their quote submission, failure to do so may result in a non-compliant quote.***

This addendum is being provided in clarification to ITQ CS-20-09 released June 9, 2020.

**Please see the attached for a minor revision to the drawing where a column was added to accommodate a higher design snow load and the thickness of the plywood to be increased to 3/4 inch.**

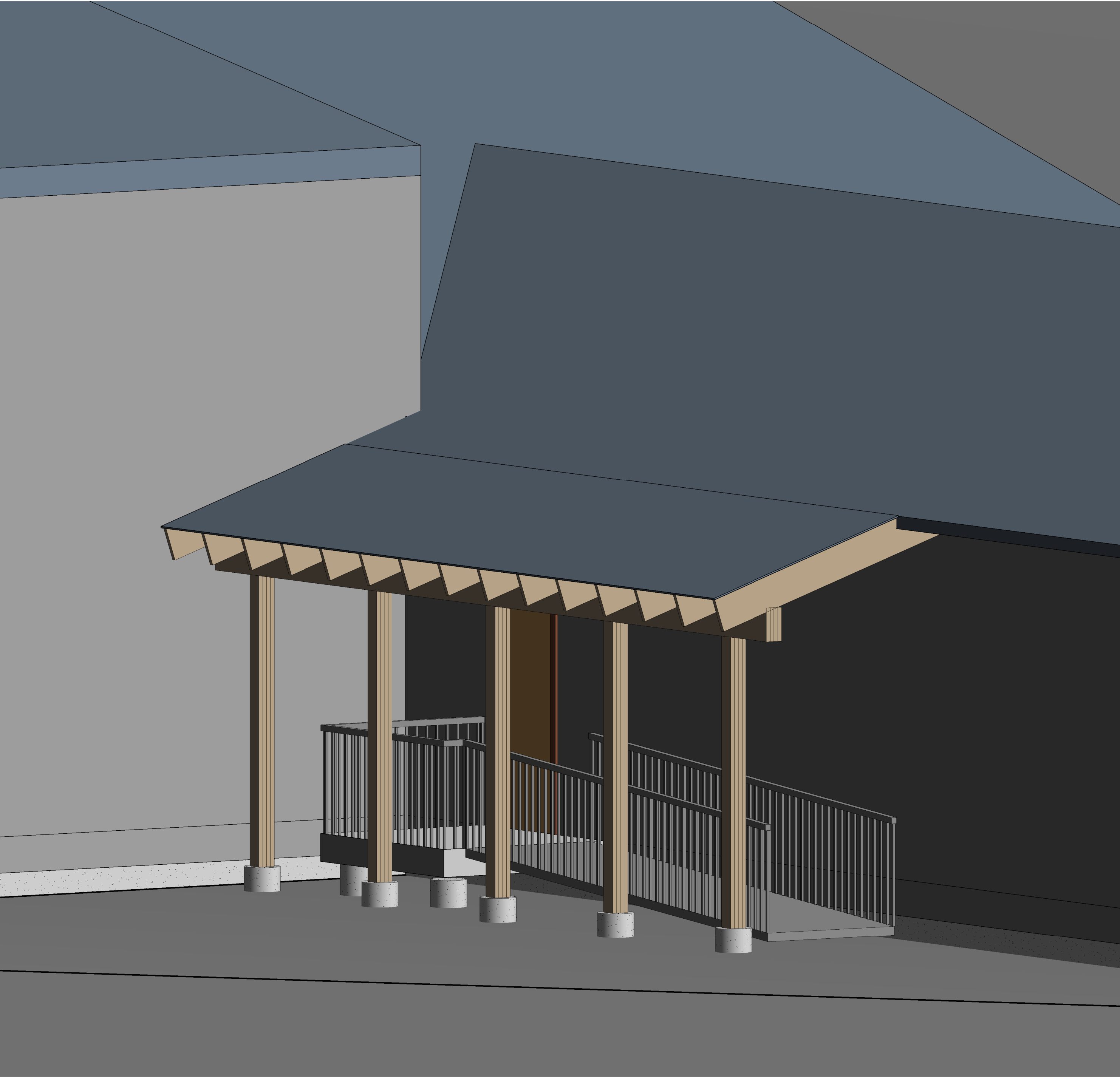
I/We hereby verify that we have considered this addendum in our quote submission.

\_\_\_\_\_  
Bidder's Signature

\_\_\_\_\_  
Date

All inquiries relating to ITQ CS-20-09 must be emailed to:  
Meredith Burmaster, Manager Community Services  
[mburmaster@rdffg.bc.ca](mailto:mburmaster@rdffg.bc.ca)

## Ness Lake Community Hall - Rear Roof Extension



### 1 3D View

DESIGN CRITERIA:

- |  |  |   |
|--|--|---|
| 1.   | DESIGN STANDARDS:  |   |
|  | BUILDING:<br>CONCRETE:<br>WOOD:  | BRITISH COLUMBIA BUILDING CODE 2018<br>CSA-A23.3-14<br>CSA-O86-14 |
| 2.   | CLIMATE & SITE DATA:   |   |
|  | GROUND SNOW LOAD (1/50):<br>ASSOCIATED RAIN LOAD (1/50):   | 4.3 kPa (90 psf)<br>0.2 kPa (4.2 psf)                             |
|  | IMPORTANCE CATEGORY:   | 1.0   |
| 3.   | DESIGN LOADS:  |   |
|  | ROOF SNOW DRIFT LOADS:<br>LENGTH OF DRIFT ( $X_d$ ):   | 7.8-2.9 kPa (163-61 psf)<br>6.7 m (22 ft)                         |
| 4.   | BEARING CAPACITY:  |   |
|  | ALLOWABLE BEARING PRESSURE:  | 100 kPa (2,000 psf)   |
| <p>THE DESIGN IS BASED ON AN ASSUMED BEARING CAPACITY. THE ENGINEER SHALL BE NOTIFIED IF THE ENCOUNTERED BEARING CAPACITIES ARE LESS THAN THE VALUES LISTED ABOVE.</p> |  |   |
| 5.   | THE CONTRACTOR SHALL ENSURE THAT CONSTRUCTION LOADS DO NOT EXCEED THE DESIGN LOADS LISTED ABOVE. |   |

GENERAL:

1. CONSTRUCTION SHALL COMPLY WITH THE CODES AND STANDARDS LISTED ON THE DRAWINGS AS WELL AS ALL APPLICABLE FEDERAL, PROVINCIAL AND MUNICIPAL REGULATIONS AND BYLAWS.
2. THE CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS BEFORE COMMENCING ANY WORK AND NOTIFY THE ENGINEER OF ANY ERRORS OR OMISSIONS.
3. ONLY USE WRITTEN DIMENSIONS. DO NOT SCALE OFF THE DRAWINGS.
4. DRAWINGS SHALL NOT BE USED FOR CONSTRUCTION UNLESS MARKED ISSUED FOR CONSTRUCTION (IFC) AND SEALED BY A PROFESSIONAL ENGINEER.

FIELD REVIEWS:

1. THE ENGINEER SHALL BE NOTIFIED OF THE CONSTRUCTION SCHEDULE IN ORDER TO SCHEDULE FIELD REVIEWS. IF THE ENGINEER IS NOT AFFORDED THE OPPORTUNITY TO REVIEW THE STRUCTURAL WORKS PRIOR TO CONCEALMENT, THEN FINAL CERTIFICATION OF THE PROJECT WILL NOT BE ISSUED.
2. THE ENGINEER SHALL BE NOTIFIED AT LEAST 24 HOURS IN ADVANCE FOR INSPECTION AND APPROVAL OF THE FOLLOWING:  
  
FOUNDATION SOILS, BEFORE BACKFILLING OR CONCRETING  
WOOD FRAMING AND ROOF SHEATHING, BEFORE CONCEALMENT
3. FIELD REVIEWS ARE PROVIDED ONLY FOR THE WORK SHOWN ON THE STRUCTURAL DRAWINGS PREPARED BY THE ENGINEER. REVIEWS ARE PERIODIC AND AT THE PROFESSIONAL JUDGEMENT OF THE ENGINEER TO DETERMINE THAT THE WORK IS IN GENERAL CONFORMANCE WITH THE DRAWINGS AND CONTRACT DOCUMENTS, AND TO FACILITATE COMPLETION OF THE LETTERS OF ASSURANCE REQUIRED BY THE AUTHORITY HAVING JURISDICTION (AHJ).
4. FIELD REVIEWS SHALL NOT RELIEVE THE CONTRACTOR OF THEIR RESPONSIBILITY AND OBLIGATION TO COMPLY WITH THE DRAWINGS AND CONTRACT DOCUMENTS. QUALITY CONTROL REMAINS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
5. ADDITIONAL FIELD REVIEWS THAT ARE REQUIRED DUE TO DEFICIENT OR INCOMPLETE WORK SHALL BE AT THE CONTRACTOR'S EXPENSE.

## FOUNDATIONS:

1. THE CONTRACTOR SHALL VERIFY THE LOCATION OF EXISTING STRUCTURES AND UTILITIES PRIOR TO EXCAVATION AND ENSURE THEY ARE PROTECTED DURING CONSTRUCTION.
2. IF ANY FOOTING DEPTHS OR ELEVATIONS ARE SHOWN, THEY ARE FOR BIDDING PURPOSES ONLY, AND ARE NOT FINAL. DEPTHS AND ELEVATIONS MAY VARY DUE TO SITE CONDITIONS.
3. SOIL SURFACES FOR BEARING SHALL BE PROTECTED AGAINST FREEZING PRIOR TO AND AFTER FOOTINGS ARE PLACED.

### CAST-IN-PLACE CONCRETE:

1. CONCRETE SHALL BE MIXED, PLACED, FINISHED AND CURED IN ACCORDANCE WITH CSA-A23.1.
2. CONCRETE SHALL BE NORMAL WEIGHT CONCRETE, MIXED USING TYPE GU CEMENT, AND CONTAINING MAXIMUM 20 mm (3/4") AGGREGATE UNLESS NOTED OTHERWISE. CONCRETE SHALL CONFORM TO THE CONCRETE SPECIFICATION TABLE BELOW:


ELEMENT	EXPOSURE	STRENGTH	MAX W/C	SLUMP	AIR	CURING TYPE
FOOTINGS	N	25 MPa (3,600 psi)	0.55	75 - 100 mm (3 - 4")	0%	1
PEDESTALS	F-2	25 MPa (3,600 psi)	0.55	75 - 100 mm (3 - 4")	4 - 7%	1

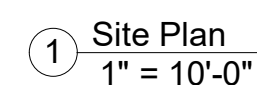
3. THE USE OF ADMIXTURES OTHER THAN AIR ENTRAINMENT, STANDARD WATER REDUCERS OR SUPER PLASTICIZERS IS NOT PERMITTED UNLESS SPECIFIED OR AUTHORIZED BY THE ENGINEER.
4. WATER SHALL NOT BE ADDED TO THE CONCRETE AFTER LEAVING THE BATCH PLANT.
5. CONCRETE SHALL BE CONSOLIDATED USING MECHANICAL VIBRATORS.
6. CONCRETE CURING TYPE 1 SHALL BE CURED FOR A MINIMUM OF 3 DAYS OR UNTIL THE CONCRETE HAS REACHED 40% OF ITS DESIGN STRENGTH. CONCRETE CURING TYPE 2 SHALL BE CURED FOR A MINIMUM OF 7 DAYS OR UNTIL THE CONCRETE HAS REACHED 70% OF ITS DESIGN STRENGTH. SEE THE CONCRETE SPECIFICATION TABLE ON THIS DRAWING FOR SPECIFIED CURING TYPES.

### REINFORCING STEEL:

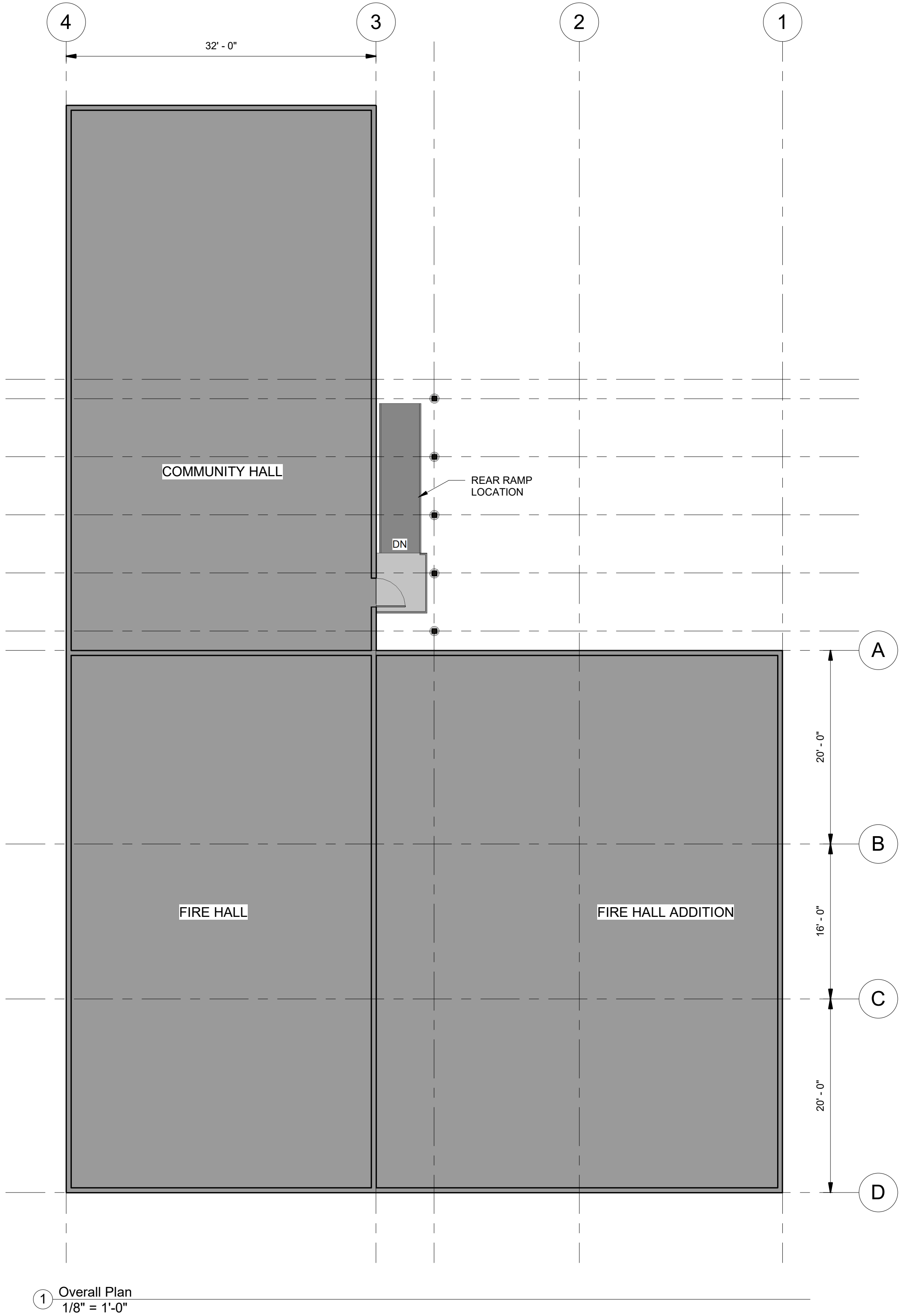
- REINFORCING STEEL SHALL BE DETAILED, FABRICATED AND PLACED IN ACCORDANCE WITH CSA-A23.1 AND THE RISC MANUAL OF STANDARD PRACTICE.
- REINFORCING STEEL SHALL CONFORM TO THE FOLLOWING UNLESS NOTED OTHERWISE:
- REINFORCING STEEL: CSA-G30.18 GRADE 400
- REINFORCING STEEL SHALL NOT BE WELDED UNLESS SPECIFIED OR AUTHORIZED BY THE ENGINEER. WELDING SHALL CONFORM TO CSA-W186.
- REINFORCING STEEL SHALL BE CLEAN AND FREE OF MUD, OIL, EXCESSIVE RUST, MILLS SCALE OR DAMAGE.
- REINFORCING STEEL SHALL BE ACCURATELY PLACED, SECURED, AND SUPPORTED TO ENSURE PROPER CONCRETE COVER AND SPACING WITHIN ALLOWABLE TOLERANCES BEFORE AND DURING CONCRETING. REINFORCING STEEL IN SLABS SHALL BE SUPPORTED BY SUITABLE SUPPORTS AT MAXIMUM 1.2 m (4'-0") ON CENTRE. BAR SUPPORTS SHALL BE MADE OF PRECAST CONCRETE, PLASTIC OR STEEL.
- PROVIDE CLEAR CONCRETE COVER FOR REINFORCING STEEL IN CAST-IN-PLACE CONCRETE AS FOLLOWS, UNLESS NOTED OTHERWISE:
- |                                       |                |
|---------------------------------------|----------------|
| CONCRETE CAST AGAINST GROUND:         | 75 mm (3")     |
| EXPOSED TO CHLORIDES/MANURE:          | 60 mm (2 3/8") |
| EXPOSED TO FREEZING/THAWING/SULPHATE: | 40 mm (1 1/2") |
| NOT EXPOSED:                          | 30 mm (1 1/8") |
- REINFORCING STEEL SHALL BE PLACED WITHIN THE FOLLOWING TOLERANCES:
- |                               |                                     |
|-------------------------------|-------------------------------------|
| BAR SPACING:                  | ± 30 mm (1 1/8")                    |
| LOCATION OF BAR ENDS & BENDS: | ± 50 mm (2")                        |
|                               | ± 20 mm (3/4") AT DISCONTINUED BARS |
| CONCRETE COVER:               | ± 12 mm (1/2")                      |
- CONCRETE COVER SHALL NOT BE REDUCED BY MORE THAN 1/3 OF THE SPECIFIED COVER.
- REINFORCING STEEL SHALL BE BENT WITH THE FOLLOWING DIAMETERS UNLESS NOTED OTHERWISE:
- |      |                |
|------|----------------|
| 10M: | 70 mm (2 3/4") |
| 15M: | 100 mm (4")    |
- HOOKS SHALL BE 90° BENDS WITH AN EXTENSION OF AT LEAST 12 BAR DIAMETERS, OR 180° BENDS WITH AN EXTENSION OF AT LEAST 4 BAR DIAMETERS, BUT NO LESS THAN 60 mm (2 3/8"). HOOKS FOR STIRRUPS AND TIES SHALL BE 135° BENDS WITH AN EXTENSION OF AT LEAST 4 BAR DIAMETERS, BUT NO LESS THAN 60 mm (2 3/8").
- REINFORCING STEEL SHALL BE CONTINUOUS, AND ADEQUATELY LAPPED AT SPICES. MINIMUM LAP LENGTHS SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE:
- |      |                |
|------|----------------|
| 10M: | 490 mm (1'-7") |
| 15M: | 740 mm (2'-5") |
- THE LAP LENGTHS ARE CONSERVATIVE FOR MOST CASES. SHORTER LAP LENGTH MAY BE USED DEPENDING ON CONCRETE STRENGTH, LOCATION OF BARS AND TYPE OF SPLICE IF AUTHORIZED BY THE ENGINEER.
- REINFORCING STEEL DOWELS SHALL NOT BE WET SET UNLESS AUTHORIZED BY THE ENGINEER.

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PB	2020-07-02	ISSUED FOR TENDER	TRM	SW	SF	
PA	2020-02-25	ISSUED FOR REVIEW	TRM	SW	SF	
PA	2019-09-06	ISSUED FOR REVIEW	TRM	SW	SF	
Rev	Date	Description	Drawn	Design	App'd	

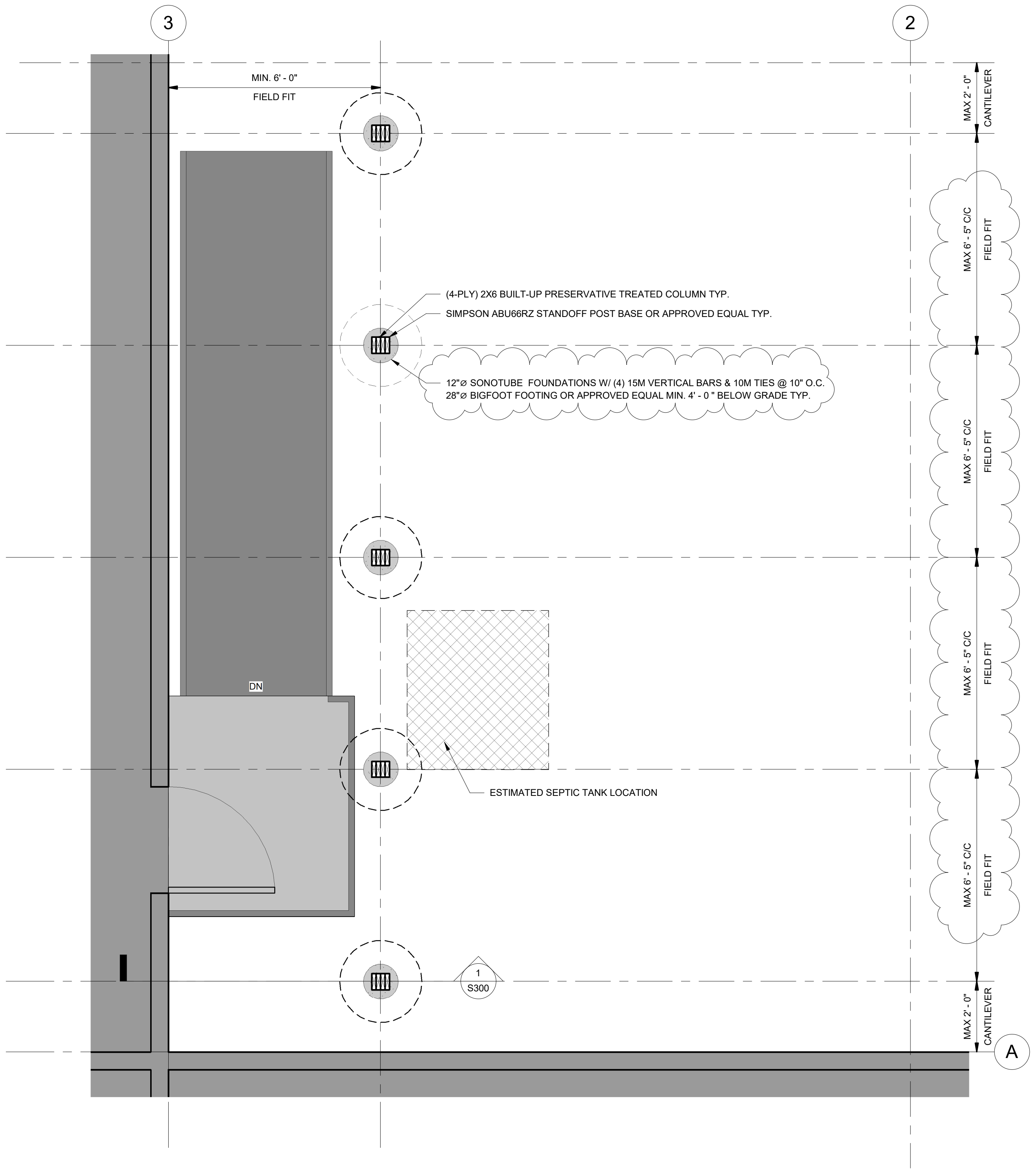
 <b>McElhanney</b>	Suite 12 556 North Nechako Rd Prince George BC Canada V2K 1A1 Tel 250 561 2229	Approved Sealed	<b>Regional District of Fraser-Fort George</b> Ness Lake Community Hall - 24205 Ness Lake Rd, Prince George BC		Drawing No.  <div style="font-size: 2em; font-weight: bold;">S000</div>
			<b>Rear Roof Extension Cover &amp; General Notes</b>		Project No. 2341-02750-06

[illegible]






1 Overall Plan  
1/8" = 1'-0"



2 Foundation Plan  
1/2" = 1'-0"

Rev	Date	Description	Drawn	Design	App'd
0	2020-07-02	ISSUED FOR TENDER	TRM	SW	SF
PB	2020-02-25	ISSUED FOR REVIEW	TRM	SW	SF
PA	2019-09-06	ISSUED FOR REVIEW	TRM	SW	SF

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Suite 12  
556 North Nechako Rd  
Prince George BC  
Canada V2K 1A1  
Tel 250 561 2229

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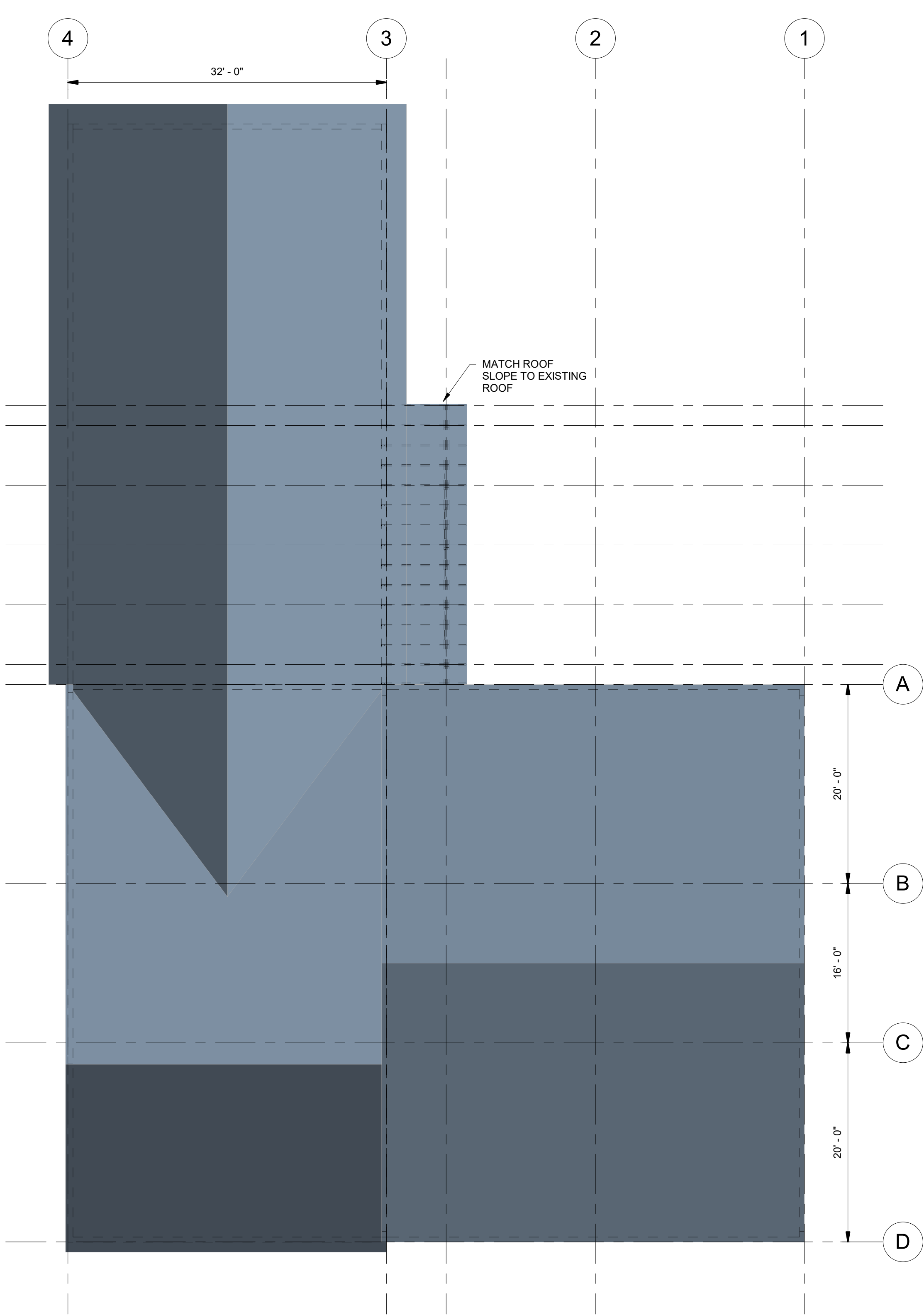
**Regional District of Fraser-Fort George**  
Ness Lake Community Hall - 24205 Ness Lake Rd, Prince George BC

**Rear Roof Extension  
Floor Plans**

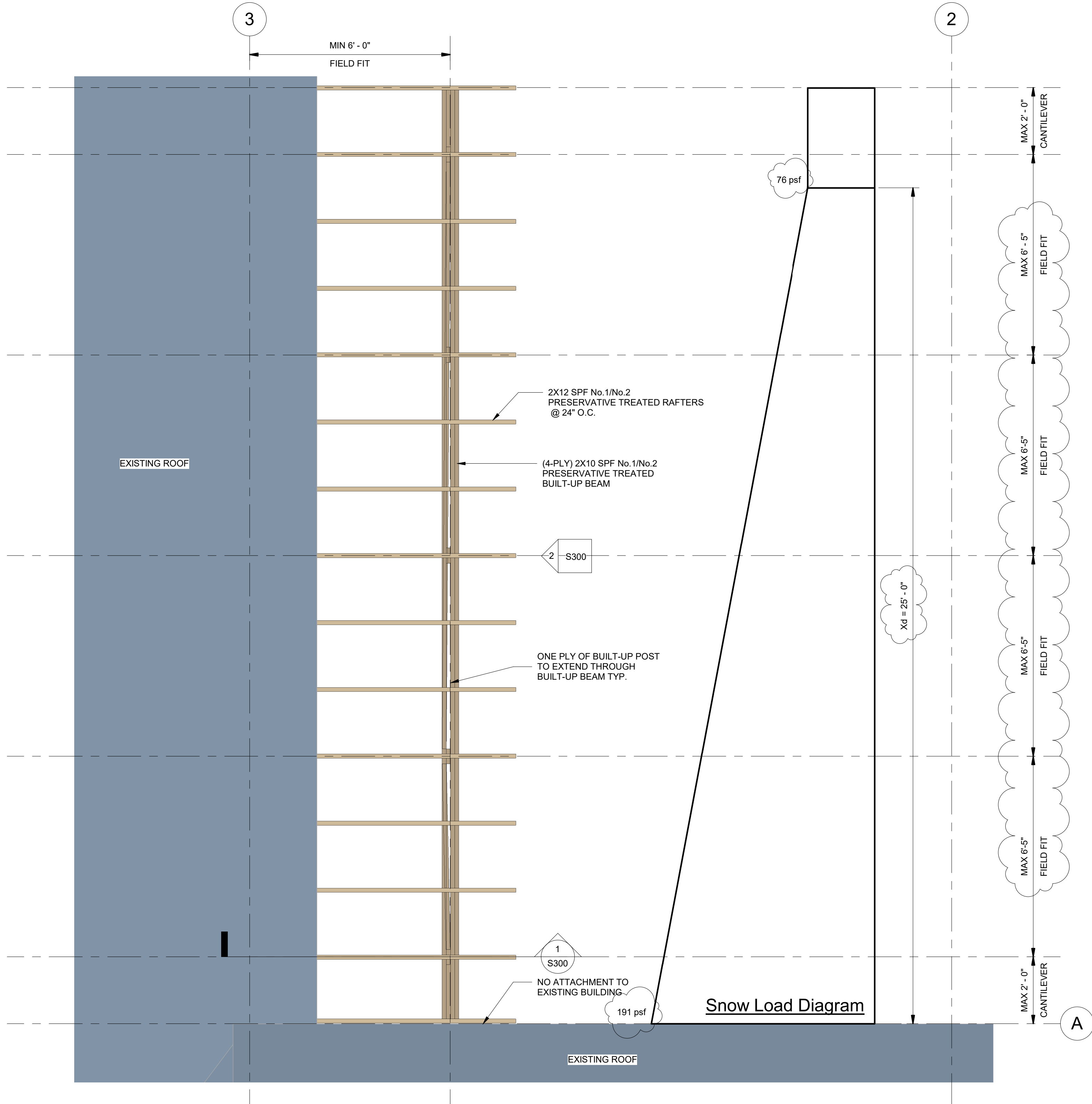
Drawing No.  
**S100**

Project No.  
2341-02750-06

Rev.  
0



① Overall Roof Plan  
1/8" = 1'-0"



② Roof Framing Plan  
1/2" = 1'-0"

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**McElhanney**

Suite 12  
556 North Nechako Rd  
Prince George BC  
Canada V2K 1A1  
Tel 250 561 2229

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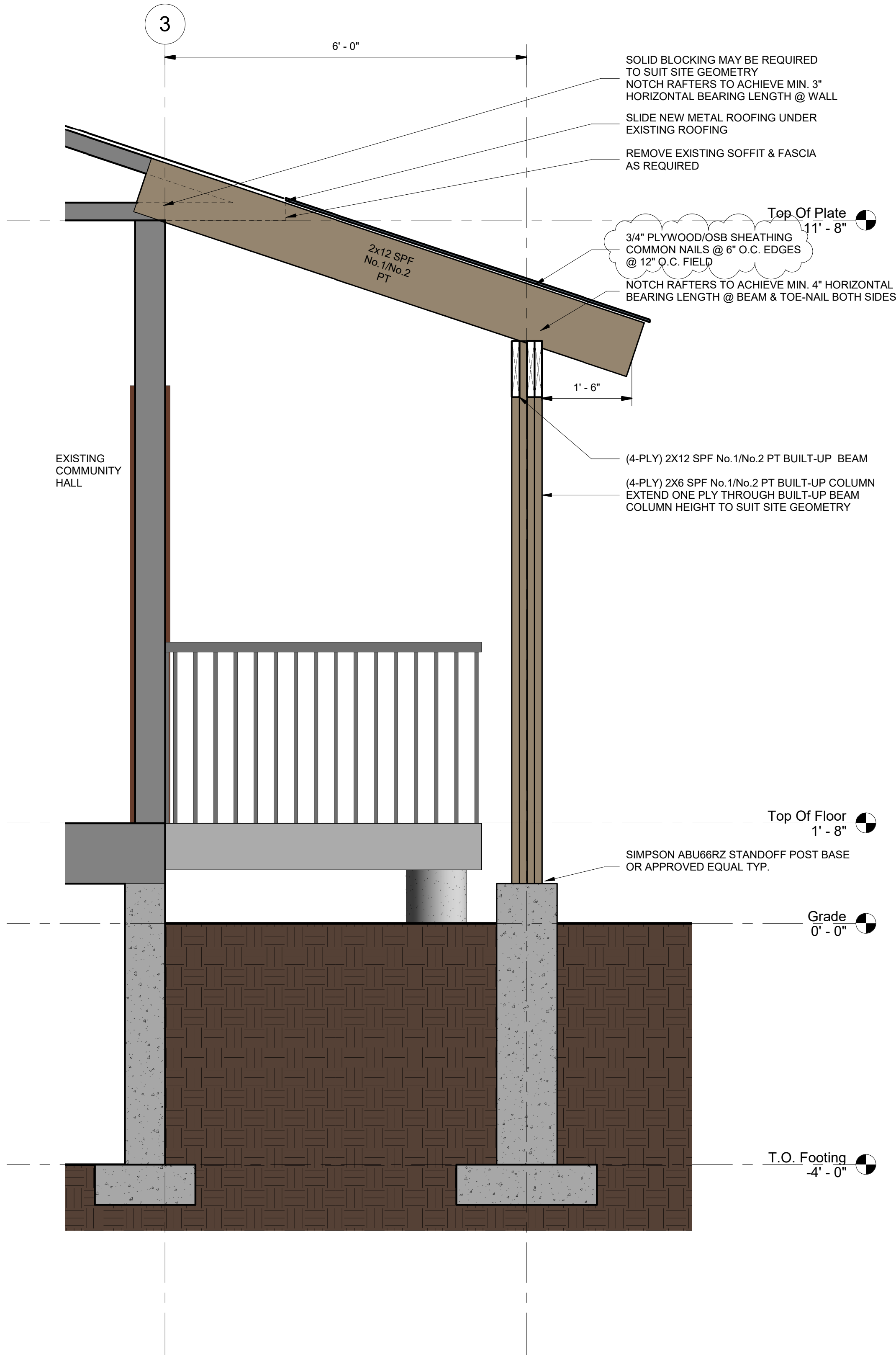
**Rear Roof Extension  
Roof Plans**

Drawing No.

**S101**

Project No.  
2341-02750-06

Rev.  
0



1 Section  
3/4" = 1'-0"



2 Elevation  
1/2" = 1'-0"

Rev	Date	Description	Drawn	Design	App'd
0	2020-07-02	ISSUED FOR TENDER	TRM	SW	SF
PB	2020-02-25	ISSUED FOR REVIEW	TRM	SW	SF
PA	2019-09-06	ISSUED FOR REVIEW	TRM	SW	SF

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**McElhanney**

Suite 12  
556 North Nechako Rd  
Prince George BC  
Canada V2K 1A1  
Tel 250 561 2229

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Ness Lake Community Hall - 24205 Ness Lake Rd, Prince George BC

**Rear Roof Extension  
Detail Views**

Drawing No.

**S300**

Project No.

2341-02750-06

Rev.

0